

AKA 1620 NW 6 CT

EASTCOAST TESTING & ENGINEERING, INC.

4100 NORTH POWERLINE ROAD - SUITE G-1
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BROWARD (954) 9727645 (SOIL) - DADE (305) 9474768

4361 OKEECHOBEE BLVD., - SUITE A-5
WEST PALM BEACH, FLORIDA 33409
1-800-3297645 (SOIL) - (561) 4718220

FACSIMILE # (954) 971-8872
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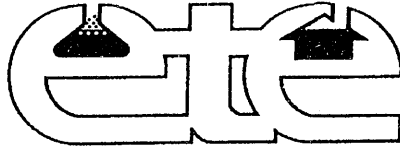
APRIL 16, 2009

**REPORT OF GEOTECHNICAL EXPLORATION &
ENGINEERING ANALYSIS / RECOMMENDATIONS**

FOR

**CITY OF FORT LAUDERDALE
COMMUNITY REDEVELOPMENT AGENCY**

**PROPOSED RESIDENCE
1621, 1615 & 1609 SISTRUNK BOULEVARD
PARCELS 5042-04-12-0050, 0040 & 0030
FORT LAUDERDALE, FLORIDA
BROWARD COUNTY, FLORIDA**



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April 16, 2009

City of Fort Lauderdale
Community Redevelopment Agency
101 NE 3rd Avenue #300
Fort Lauderdale, Florida 33301

Attention: Bob Wojcik

Reference: Geotechnical Investigation for Proposed One-Four Story Structure
1621, 1615, &1609 Sistrunk Boulevard

Gentlemen;

Eastcoast Testing and Engineering, Inc. has performed as requested the geotechnical exploration for the subject project in general accordance with our proposal dated March 18th, 2009.

It is our understanding that this subsoil investigation was performed to assist in the formulation of foundation assumptions for a proposed 1-4 story reinforced concrete residential structure. We have prepared general foundation recommendations based on the subsoil data obtained from our borings as well as our experience with projects of similar scope and size. Upon the completion of the design process, further geotechnical engineering and testing services may be necessary to finalize foundation preparation procedures and/or certification for the proposed structure.

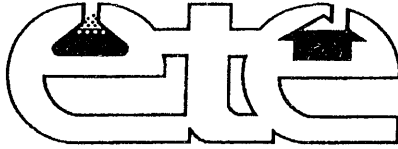
Thank you for allowing us to be of service to you in this matter. If you have any further questions, please do not hesitate to contact our office.

Respectfully Submitted
EASTCOAST TESTING AND ENGINEERING, Inc.,
Certificate of Authorization #3425

Craig S. Smith, President

Etienne Prophete, V.P., P.E.
State of Florida #44316

WPD/CFL/2900771LETTER/CS



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ESTABLISHED 1981

April 16, 2009

**Report of Engineering Evaluation For : City of Fort Lauderdale Community Development
Project : Proposed New Residence
Location: 1621, 1615, & 1609 Sistrunk Boulevard
Fort Lauderdale, Florida
Broward County, Florida**

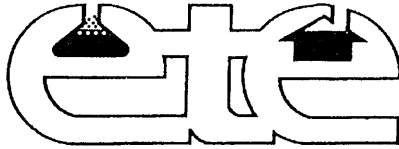
Gentlemen;

As per your request EastCoast Testing & Engineering, Inc. performed the standard penetration test borings at the above reference location as requested. The purpose of this investigation was to provide information concerning the site and subsurface conditions in order to provide site preparation and foundation design recommendations for support of the proposed construction. This report presents our findings and foundation recommendations.

We understand that plans and information with regards for this project consist of a new one story to possible four story residential structure to be constructed of conventional concrete and load bearing masonry reinforced walls, (cmu). At the time our subsurface investigation the site was vacant with one existing structure to the east of the property. We assume the site will not require more than one to three feet of engineered construction fill to reach final grade. Major intersections are immediately north of Sistrunk Boulevard and east of NW 27th Avenue in Fort Lauderdale, Florida, Broward County, Florida

A review of our boring logs shows the upper level of subsoils are generally comprised of loose to medium dense fine to medium grained sands with some concrete debris, little limerock fragments and roots to a depth of +/-1.5 feet below the surface grade. Below these subsoils our borings disclosed stratum of fine to medium grained sands and fine to medium grained sands with traces of limerock fragments in a loose to medium dense compaction condition to a depth of +/-4.0 feet below grade. Below these subsoils our borings discovered fine to medium grained sands in a very loose to loose state of relative consolidation which continued to +/-8.0 feet below the existing surface grade elevation. Below this zone were multifarious layers of fine to medium grained sands and fine to medium grained sands with some limerock fragments in a loose to medium dense compaction state which extended to -22.0 feet below the surface. From this depth our borings found stratum of fine to medium grained sands and fine to medium grained sands with traces of limerock fragments predominantly in a dense compaction condition which extended to -32.0 feet below the surface grade. These sand substrate continue past the terminal depth of our subsurface exploration, thirty-two feet maximum penetration.

The results of our subsurface study confirm that the site is generally favorable for shallow foundations using geotechnical considerations. The proposed structure may be supported on shallow foundations. Should the structure, however be designed for foundations proportioned for bearing capacities in excess of 2500 psf alternate deep foundation methods are recommended. Detailed recommendations follow in the remaining sections of the report.



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Page 2. Order #2900771

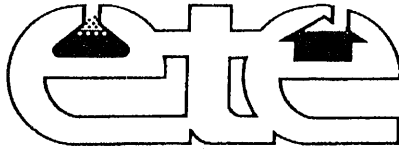
In order to prepare the site to support construction designed for an allowable soil bearing capacity of 2500 psf, we recommend the following procedures be implemented.

1. Clear, grub and excavate, (undercut) the proposed building pad area to a minimum depth of 4.0 feet plus an extended perimeter of five feet beyond any proposed wall footing or foundation element. Care should be taken so as to insure the complete removal of any deleterious materials encountered, including any construction debris, concrete, organics, grass, rubble, vegetation, stumps, roots, foreign material, debris, silts, clays or muck.

2. Once this has been accomplished and inspected by a geotechnical representative from this office or by the engineer compact the bottom of the excavated area with twenty (perpendicular) overlapping passes in each of two directions with a large single Smooth Drum Static Compactor, (10-14 Tons, Bomag BW11AS or equivalent) until the bottom 24" of the excavation have been compacted in excess of 98% of the material's modified maximum dry density as per AASHTO T-180. Field density-moisture tests and/or cone penetrometer tests shall be performed to verify the resultant compaction effort prior to the next phase of construction. Densification shall continue until no further settlement can be visually discerned at the subgrade surface. Each pass of the roller should overlap the preceding pass by one half of the width of the vibratory drum. Maximum speed of the roller should be 2 ft./second. For optimum compaction efficiency we recommend that the soils be within +/-2% of optimum moisture at the time of densification. Care must be exercised by the contractor during the excavation and compaction phase of construction. The close proximity of the existing foundations to the construction boundaries necessitate that he utilize what ever means necessary to insure the stability of the existing foundations during construction. De-watering devices such as well points or sump pumps may be necessary due to the existence of a high ground water table elevation

3. Once this has been achieved and verified by our office, the proposed building pad may be brought to construction grade utilizing clean granular fill. Suitable materials excavated and stockpiled at the site may be reused for backfill within the foundation after tested for approval by the Engineer. Engineered fill shall be tested for suitability, classification and proctor analysis as required for certification purposes. The construction fill may be placed in lifts not to exceed twelve inches in compacted thickness. Each lift should be compacted to a minimum density of 98% of the material's modified proctor density as per AASHTO T-180. Clean granular fill shall be construed to mean granular material containing no more than 5% by weight organic and clayey matter and no man-made debris. It shall be free of vegetation, foreign material, roots, fiber, branches, leaves, and should not contain any rock and gravel larger than 3 inches or 50% of the compacted layer thickness.

4. Suitable materials for uses as backfill consist of the following; GW, (well graded gravel and well grade gravel with sand), GP, (poorly grade gravel and poorly graded gavel with sand), SW, (well graded sand) and SP, (poorly graded sand). Following and during the clearing and excavation stage the area shall be witnessed by an inspector from this laboratory for approval prior to in-situ densification.



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The excavated surface and each 12" compacted lift of on-site or imported fill material within the footing and slab areas shall be tested to within 98% of the Soils Modified Maximum Dry Density as per AASHTO DESIGNATION T-180 inspected by this laboratory and verified with field density-moisture relationships. No vibration shall take place in the vicinity of any existing structure as this may cause localized damage to nearby existing structures.

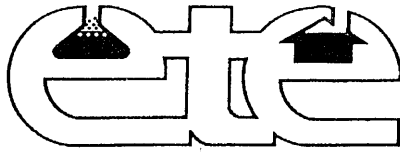
Footing embedment shall be of sufficient depth below the adjacent grade so as to comply with all local and area building codes. Minimum footing widths of 18" and 36" are recommended for continuous wall footings and individual column pads respectively, although they may not develop the full allowable bearing pressures. To assure a safety factor against bearing capacity failure, the foundation bottoms should be placed a minimum of 18 inches below the lowest adjacent grade. Foundation elements may be designed as isolated footings or as a monolithic type of foundation/slab system, as long as ample consideration is given to the increased shear stresses inherent in monolithic systems at the slab to footing interface. Surface compaction specifications shall be verified utilizing in place density tests at the frequency of 1 test per 100 lf of wall footing, and 1 test per column pad. Slab areas and undisturbed pad lifts may be tested at the frequency of two tests per 2500 sf, but in no case less than 3 per lift. Reinforcement for footings should be proportioned to meet the requirements of ACI 318, latest edition, and other applicable codes.

Slab-on-grade construction may be employed for the ground floor of the building. Ground floor slabs can bear directly on the densified structural fill after the backfill and compaction operations have been completed. Proper joints shall be provided at the junctions of the slabs with the building walls and columns so that a small amount of independent movement can occur without causing damage. A modulus of subgrade value of 150 pounds per cubic inch (pci) may be used for ground floor slab design.

Foundations designed and constructed in accordance with the recommendations of this report are expected to sustain a maximum total settlement of 1 inch and differential settlements between adjacent footings of not more than ½ inch or approximately one-half of the total settlement. Distortions that occur along the wall footings due to differential settlement should not be more than 1 in 500.

An impervious membrane should be installed between the underside of the floor slab and the soil substrate to serve as a barrier to moisture rise from the subgrade. Ordinarily, a 6 mil thick film of polyethylene is sufficient for this purpose. However, some floor coverings may have a comparatively sensitive tolerance to moisture flux that a thin polyethylene film cannot suppress. Under these conditions, other types of moisture membranes may need to be considered.

The natural ground water table was discovered at +/-4.25-4.67 feet at the time of our subsurface exploration. Fluctuation in the observed groundwater levels should be expected due to rainfall variations, seasonal climatic changes, construction activity and other on-site specific factors.



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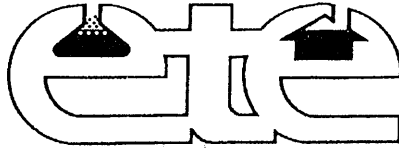
VIBRO-FLOATATION ALTERNATIVE

As an alternative to shallow foundation system we recommend to stabilize the very loose to loose soils with a system of vibro-floatation. In order to minimize the differential settlement of large structures with foundation widths in excess of 30-48 inches, we recommend that the very loose subsoils noted between the surface and +/-12 feet be densified utilizing Vibro-Floatation compaction. Vibro-floatation densified loose subsurface soils utilizing a vibrating probe inserted through the loose subsoils. This probe is usually in excess of 18 inches in diameter and 22 feet in length. The action of the vibrator along with water jetting allows for the sand particles to be reconfigured into a denser formation.

After the loose sands are densified, the probe is raised a given distance and another layer is treated. This process is repeated until the tip of the probe reaches the surface and is withdrawn. Any voids created during the vibro-floatation process may be filled with clean granular fill. The minimum depth of treatment is usually twice the least width of the proposed foundation. Probe spacing and depth is usually the responsibility of the speciality subcontractor. In addition, the process shall be checked and inspected by a geotechnical representative of this firm during the vibro-floatation stabilization process to verify the work is being performed in accordance with project specifications. Upon completion, standard penetration test borings shall be performed to verify the resultant compaction effort to verify the resultant compaction effort to a probe point for certification purposed. After the completion of the vibro-floatation process, the surficial soils are compacted with a large vibratory roller to densify the upper 24 to 36 inches of the site. If this foundation process is chosen, all load bearing elements as well as the slab on grade may be designed as isolated column pads and continuous wall footings bearing on the improved subsoils.

It is our opinion that the vibrofloatation process will provide an assumptive allowable soil bearing capacity of 3000-4000 psf suitable for design purposes. This value may be increased to 5000 psf if clean washed gravel is substituted for the replacement material. Prior to choosing this option, it is our recommendation that a settlement analysis be performed to insure that the anticipated settlement of these spread footings are within the allowable limits. This analysis cannot be performed until the structural design process is complete, and anticipated column loads are known.

At this time specific foundation information has not been provided for us to evaluate and estimate any anticipated settlement. It is recommended that this office be provided the opportunity to make a general review of the foundation and earthwork plans and specifications presented in this report. Our report has been written in a guideline recommendation format and is not appropriate for use as a specification-type format. It is recommended that this report not be made a part of the contract documents, however, it should be made available to prospective contractors for information purposes.



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The Standard Penetration Test ASTM D-1586

The Standard Penetration Test is the most commonly employed tool utilized to identify in-situ subsurface soil conditions. The "N" values obtained from the boring provide an accurate estimation of internal soil characteristics such as relative density, internal shear strength, angle of internal friction, and the approximate range of the soil's unit weight. These "N" values represent the resistance of a 2 inch diameter split spoon sampler driven by a 140 pound hammer free falling 30 inches. Each drive of the 24 inch long split spoon is divided into four six inch increments. The second and third increments are totaled to produce the "N" value found on your report.

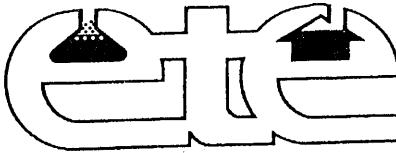
The Standard Penetration Test also allows for the recovery of soil samples which are returned to our laboratory and visually examined and classified. The SPT samples are available for laboratory testing if requested. Samples are generally held for 90 days unless otherwise directed by the client.

An approximate ground water table is obtained from the borehole upon completion of the drilling procedures. This water table is useful in the general evaluation of particular soil conditions, and may give the contractor some insight into what can be anticipated during construction. It should be noted that the ground water level will fluctuate seasonally. This level may also be affected by local drawdowns, soil conditions, and the watershed's contribution to the underlying aquifer. It should not be construed to be a measure of the soil's permeability, or of the dewatering characteristics of the site.

Although the standard penetration test is one of the most reliable methods used to identify soil characteristics and types, it may only represent a small fraction of the materials actually deposited at the site. As is common industry practice, we have assumed a uniformity of profile between borings to provide a subsoil profile for engineering purposes. This profile is strictly based on the data obtained from the borings, and if unusual or varying conditions are found we should be notified immediately.

A test is expressly representative of the immediate location tested, and the reliability of the conclusions are a direct result of the quantity of tests performed. Any variation in location may reveal similarly some changes in the depth, thickness, texture, and conditions of the stratum encountered.

Unless specifically stated otherwise, and specifically directed and prearranged by the client, all elevations are taken with respect to the existing ground surface at the time of testing. Boring locations are usually obtained in the field by pacing off distances and approximating right angles to landmarks and property corners. More precise locations may be obtained from on site surveys and placement of the boring locations by a Land Surveyor, Registered in the State of Florida. These services are provided at additional costs and are beyond the scope of this report.



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The data presented herein was obtained for the specific purposes stated in this report, and should not be misconstrued to apply to any other circumstance, project, or ancillary use unless so specified and addressed by the engineer of record.

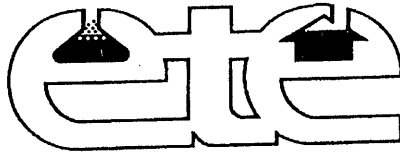
Thank you for using EASTCOAST TESTING AND ENGINEERING, INC., for your geotechnical needs. Should you need further assistance with this or any other project, please contact this office.

Respectfully Submitted;
EASTCOAST TESTING & ENGINEERING, INC.
Certificate of Authorization #3425


4-16-09
Etienne Prophete V.P., P.E. #44316


Craig S. Smith, President

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TEST BORING REPORT

CVO: _____	C.O.D.: _____	PO#: _____
LABORATORY NUMBER: <u>2900771A</u>	OFFICE FAX #: <u>954-971-8872</u>	BORING NUMBER: <u>1</u>
CLIENT: <u>CITY OF FT. LAUDERDALE COMMUNITY REDEVELOPMENT AGENCY</u>	CUSTOMER #: _____	CREW CHIEF: <u>HE</u>
PROJECT: <u>PROPOSED RESIDENCE</u>	DRILLER: <u>RM</u>	DRILL RIG#: <u>F-250</u>
PROJECT LOCATION: <u>1621, 1615 & 1609 SISTRUNK BLVD, PARCELS 5042-04-12-0050, 0040 & 0030</u>	DATE: <u>04/13/09</u>	ELEV: <u>N/A</u>
BORING LOCATION: <u>APPROX. 20' NORTH & 60' WEST OF SE PROPERTY CORNER (1609)</u>	GROUND WATER: <u>4'8"</u>	CASING: <u>SPT</u>

NOTE: SURVEY NOT GIVEN UNLESS NOTED: B.E.G: BELOW EXISTING GRADE LOCATIONS ARE APPROX UNLESS STAKED

DEPTH FEET	SAMPLE NUMBER	BORING NUMBER: 1	PAGE NUMBER: 1	N VALUES	BLOWS ON	SPT
1	1	Gray fine-medium grained sand, trace of limerock fragments and trace of root	[0-1.5']	8	3	5
2					3	3
3	2	Tan fine-medium grained sand	[1.5'-4']	8	4	4
4					4	4
5	3	Dark brown fine-medium grained sand	[4'-8']	2	1	1
6					1	0
7					0	1
8				1	0	0
9	4	Tan fine-medium grained sand and some limerock fragments	[8'-12']	11	2	4
10					7	5
11					5	5
12				9	4	4
13	5	Light tan fine-medium grained sand and some limerock fragments	[12'-16']	16	6	7
14					9	11
15					10	10
16		BORING TERMINATED AT 16'		19	9	9

STANDARD PENETRATION TEST BORING: BLOWS PER FOOT ON 2" O.D. SAMPLER WITH 140 LB. HAMMER FALLING 30"

SOIL INVESTIGATION AND SAMPLING BY AUGER BORINGS: A.S.T.M. D 1452/STANDARD PENETRATION TEST: ASTM D1586.

THE ABOVE TEST BORING WAS CONDUCTED IN ACCORDANCE WITH A.S.T.M. DESIGNATION D-1586.

THE SAMPLES COLLECTED CONSTITUTE A MINUTE PERCENTAGE OF THE SUBSOILS AT THE SITE. AS A MUTUAL PROTECTION THE SOILS WILL BE STORED IN OUR LABORATORY FACILITIES FOR A MAXIMUM OF THREE (3) MONTHS. THE OWNER, ARCHITECT AND/OR ENGINEER ARE ENCOURAGED TO VISUALLY INSPECT SAMPLES PRIOR TO PURCHASE OF PROPERTY AND DESIGN OF THE STRUCTURE.

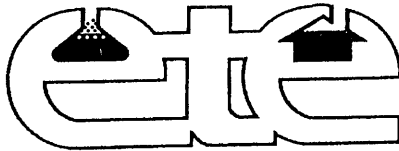
RESPECTFULLY SUBMITTED,
EASTCOAST TESTING & ENGINEERING, INC.,
C.A. # 3425

ETIENNE PROPHETE, P.E.
STATE OF FLORIDA #44316

CRAIG SMITH, PRESIDENT

THE INTENT OF THIS REPORT IS NOT FOR ENVIRONMENTAL PURPOSES UNLESS SPECIFIED.
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THRESHOLD/SPECIAL INSPECTIONS, BORINGS, DENSITY, ASPHALT, CONCRETE & "GEOTECHNICAL" TESTING LABORATORY



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TEST BORING REPORT

CVO: _____ C.O.D.: _____ PO#: _____
 LABORATORY NUMBER: 2900771B OFFICE FAX #: 954-971-8872 BORING NUMBER: 2
 CLIENT: CITY OF FT. LAUDERDALE COMMUNITY REDEVELOPMENT AGENCY CUSTOMER #: _____
 PROJECT: PROPOSED RESIDENCE CREW CHIEF: HE
 PROJECT LOCATION: 1621, 1615 & 1609 SISTRUNK BLVD, PARCELS 5042-04-12-0050, 0040 & 0030 DRILLER: RM
 BORING LOCATION: APPROX. 20' SOUTH & 50' EAST OF NW PROPERTY CORNER (1621) DRILL RIG#: F-250
 GROUND WATER: 4'8" DATE: 04/13/09 ELEV: N/A CASING: SPT

NOTE: SURVEY NOT GIVEN UNLESS NOTED: B.E.G: BELOW EXISTING GRADE LOCATIONS ARE APPROX UNLESS STAKED

DEPTH FEET	SAMPLE NUMBER	BORING NUMBER: 2	PAGE NUMBER: 1	N VALUES	BLOWS ON	SPT
1	1	Gray fine-medium grained sand, some concrete fragments, little limerock fragments & trace of root [0-1.5']		18	4	8
2					10	8
3	2	Light gray fine-medium grained sand			11	12
4		[1.5'-3.5']		21	9	6
5	3	Dk brown fine-medium grained sand & tr limerock frags. [3.5'-4.5']			4	4
6	4	Dark brown fine-medium grained sand		6	2	3
7		[4.5'-8']			2	2
8				5	3	2
9	5	Tan fine-medium grained sand and some limerock fragments			3	2
10		[8'-13']		5	3	1
11					3	5
12				11	6	8
13					9	9
14	6	Tan fine-medium grained sand		15	6	8
15		[13'-16']			7	10
16		BORING CONTINUED		20	10	10

STANDARD PENETRATION TEST BORING; BLOWS PER FOOT ON 2" O.D. SAMPLER WITH 140 LB. HAMMER FALLING 30"

SOIL INVESTIGATION AND SAMPLING BY AUGER BORINGS: A.S.T.M. D 1452/STANDARD PENETRATION TEST: ASTM D1586.

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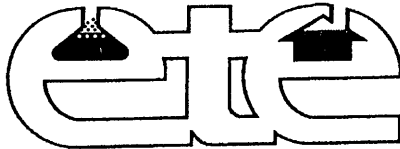
RESPECTFULLY SUBMITTED,
EASTCOAST TESTING & ENGINEERING, INC.,
C.A. # 3425

ETIENNE PROPHETE, P.E.
STATE OF FLORIDA #44316

CRAIG SMITH, PRESIDENT

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THRESHOLD/SPECIAL INSPECTIONS, BORINGS, DENSITY, ASPHALT, CONCRETE & "GEOTECHNICAL" TESTING LABORATORY



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CVO: _____ C.O.D.: _____ PO#: _____
 LABORATORY NUMBER: 2900771B1 OFFICE FAX #: 954-971-8872 BORING NUMBER: 2
 CLIENT: CITY OF FT. LAUDERDALE COMMUNITY REDEVELOPMENT AGENCY CUSTOMER #: _____
 PROJECT: PROPOSED RESIDENCE CREW CHIEF: HE
 PROJECT LOCATION: 1621, 1615 & 1609 SISTRUNK BLVD, PARCELS 5042-04-12-0050, 0040 & 0030 DRILLER: RM
 BORING LOCATION: APPROX. 20' SOUTH & 50' EAST OF NW PROPERTY CORNER (1621) DRILL RIG#: F-250
 GROUND WATER: 4'8" DATE: 04/13/09 ELEV: N/A CASING: SPT

NOTE: SURVEY NOT GIVEN UNLESS NOTED: B.E.G: BELOW EXISTING GRADE LOCATIONS ARE APPROX UNLESS STAKED

DEPTH FEET	SAMPLE NUMBER	BORING NUMBER: 2	PAGE NUMBER: 2	N VALUES	BLOWS ON	SPT
		VISUAL SOIL CLASSIFICATION/AASHTO M145/ASTMD2487				
17	7	Light tan fine-medium grained sand			6	6
18		[16'-20']		12	6	7
19					7	11
20				22	11	12
21	8	Brown fine-medium grained sand			6	6
22		[20'-25']		12	6	9
23					14	13
24				31	18	22
25					24	26
26	9	Brown fine-medium grained sand		28	2	7
27		[25'-28.5']			12	17
28				39	22	22
29					16	16
30	10	Tan fine-medium grained sand & trace of limerock fragments		30	14	15
31		[28.5'-32']			16	13
32		BORING TERMINATED AT 32'		25	12	11

STANDARD PENETRATION TEST BORING: BLOWS PER FOOT ON 2" O.D. SAMPLER WITH 140 LB. HAMMER FALLING 30"

SOIL INVESTIGATION AND SAMPLING BY AUGER BORINGS: A.S.T.M. D 1452/STANDARD PENETRATION TEST: ASTM D1586.
 THE ABOVE TEST BORING WAS CONDUCTED IN ACCORDANCE WITH A.S.T.M. DESIGNATION D-1586.
 THE SAMPLES COLLECTED CONSTITUTE A MINUTE PERCENTAGE OF THE SUBSOILS AT THE SITE. AS A MUTUAL PROTECTION THE SOILS WILL BE STORED IN OUR LABORATORY FACILITIES FOR A MAXIMUM OF THREE (3) MONTHS. THE OWNER, ARCHITECT AND/OR ENGINEER ARE ENCOURAGED TO VISUALLY INSPECT SAMPLES PRIOR TO PURCHASE OF PROPERTY AND DESIGN OF THE STRUCTURE.

RESPECTFULLY SUBMITTED,
 EASTCOAST TESTING & ENGINEERING, INC.,
 CA # 3425

ETIENNE PROPHETE, P.E.
 STATE OF FLORIDA #44316

CRAIG SMITH, PRESIDENT

THE INTENT OF THIS REPORT IS NOT FOR ENVIRONMENTAL PURPOSES UNLESS SPECIFIED.
 AUTHORIZATION FOR PUBLICATION OF STATEMENTS, CONCLUSIONS, CERTIFICATIONS OR EXTRACTS REGARDING THIS OR ANY OTHER TEST REPORT BY THIS FIRM, (ETE) IS STRICTLY PROHIBITED UNLESS WRITTEN AUTHORIZATION IS PROVIDED AND APPROVED BY EASTCOAST TESTING & ENGINEERING, INC.

LORI PARRISH
BROWARD
COUNTY
PROPERTY
APPRAISER



2009 Pictometry



View From:

Map Size:

Year:

Image: South (01/17/09) [Image Only](#)

